



Griffith-based analysis of crack initiation location in a Brazilian test

Yousef Navidtehrani^a, Covadonga Betegón^a, Robert W. Zimmerman^b, Emilio Martínez-Pañeda^{c,*}

^a Department of Construction and Manufacturing Engineering, University of Oviedo, Gijón 33203, Spain

^b Department of Earth Science and Engineering, Imperial College London, London SW7 2AZ, UK

^c Department of Civil and Environmental Engineering, Imperial College London, London SW7 2AZ, UK

ARTICLE INFO

Keywords:

Rock mechanics
Brazilian test
Fracture
Finite element analysis
Griffith

ABSTRACT

The Brazilian test has been extremely popular while prompting significant debate. The main source of controversy is rooted in its *indirect* nature; the material tensile strength is inferred upon assuming that cracking initiates at the centre of the sample. Here, we use the Griffith criterion and finite element analysis to map the conditions (jaws geometry and material properties) that result in the nucleation of a centre crack. Unlike previous studies, we do not restrict ourselves to evaluating the stress state at the disk centre; the failure envelope of the generalised Griffith criterion is used to establish the crack nucleation location. We find that the range of conditions where the Brazilian test is valid is much narrower than previously assumed, with current practices and standards being inappropriate for a wide range of rock-like materials. The results obtained are used to develop a protocol that experimentalists can follow to obtain a valid estimate of the material tensile strength. This is showcased with specific case studies and examples of valid and invalid tests from the literature. Furthermore, the uptake of this protocol is facilitated by providing a MATLAB App that determines the validity of the experiment for arbitrary test conditions.

1. Introduction

The Brazilian test, also known as the Splitting Tensile Strength test, is arguably the most popular laboratory experiment for estimating the tensile strength of rocks and other quasi-brittle materials.¹ It was, independently, first proposed by Carneiro² and Akazawa³ in 1943, and has been considered a standardised test since 1978, when it was included as a Suggested Method of the International Society for Rock Mechanics (ISRM).⁴ As shown in Fig. 1, the test is comprised of two loading jaws, typically made of steel, and a disc-shaped sample. The jaws are configured so as to contact the sample at diametrically-opposed surfaces. Critical variables are the jaw radius, R_j , the disk radius, R_d , the disk thickness t , the measured reaction force P , and the contact angle α .

Assuming isotropic, linear elastic material behaviour, Hondros⁵ derived an equation that relates the measured load P and contact angle α with the maximum principal stress at the centre of the disk:

$$(\sigma_1)_{x=0,y=0} = \frac{2P}{\pi R_d t \alpha} \left(\sin \alpha - \frac{\alpha}{2} \right). \quad (1)$$

Thus, from the critical values of P and α at failure, one can use Eq. (1) to estimate the material tensile strength σ_t upon assuming that the maximum value of σ_1 is attained at the centre of the disk: $\sigma_t = (\sigma_1)_{x=0,y=0}$. However, Eq. (1) is derived assuming the application of a

uniform pressure. Moreover, being able to experimentally measure the contact angle at failure is far from trivial. Consequently, standards are built upon the assumption of a zero contact angle, simplifying Eq. (1) to the case of a concentrated load:

$$(\sigma_1)_{x=0,y=0} = \frac{P}{\pi R_d t}, \quad \text{for } \alpha \rightarrow 0. \quad (2)$$

Eq. (2) is often referred to as the Hondros's point load solution or the Hertz solution.⁶ Using Eq. (2), the material tensile strength can be readily estimated from the critical load (P_c): $\sigma_t = (\sigma_1)_{x=0,y=0} = P_c / (\pi R_d t)$. However, this *indirect* approach builds upon a number of assumptions; most notably: (i) the load is assumed to be a concentrated point load, and (ii) cracking initiates from the centre of the disk. In practice, fulfilling these two assumptions depends on the choices of test geometry and material. Numerical computations show the existence of three regimes. Sufficiently low contact angles will satisfy Eq. (2) and lead to a maximum value of σ_1 at the disk centre. As the contact angle increases, Eq. (2) is no longer satisfied, but the maximum magnitude of the tensile principal stress is still attained at the centre. And finally, if the contact angle is sufficiently large then not only is Eq. (2) not satisfied but also the location of the maximum tensile stress moves away from the disk centre. Thus, the validity of the Brazilian test is sensitive to the contact angle at failure, which is itself dependent

* Corresponding author.

E-mail address: e.martinez-paneda@imperial.ac.uk (E. Martínez-Pañeda).

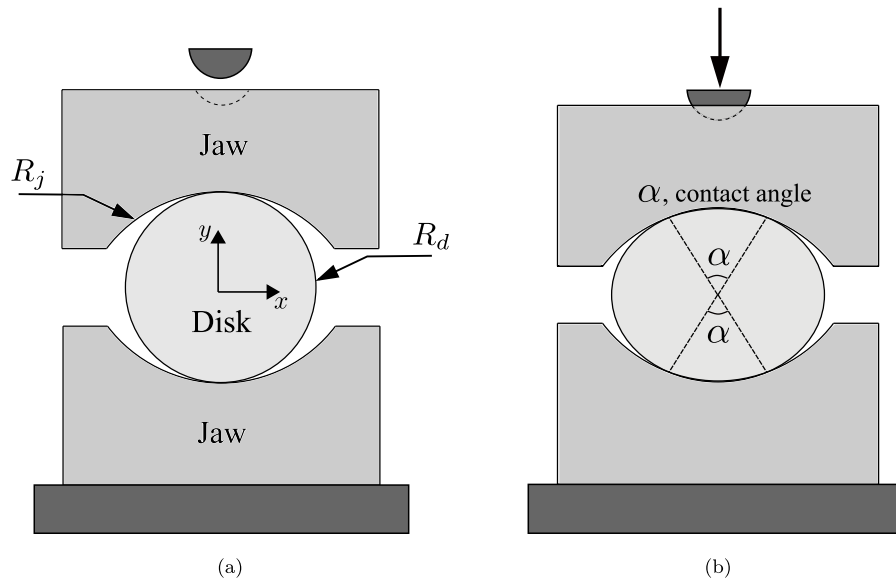


Fig. 1. Brazilian test configuration in the (a) undeformed, and (b) deformed states. The sketch shows the main variables: the jaw radius R_j , the disk radius R_d , and the contact angle at failure α . A reaction force P is measured.

on the elastic properties of the disk and jaws (Young's moduli E_d , E_j ; Poisson's ratios ν_d , ν_j), the sample and jaw radii (R_d , R_j), and the critical load (i.e., the material strength). Not surprisingly, this sensitivity to material and test parameters has fostered significant discussion in the academic literature. Despite the current popularity of the Brazilian test, early studies highlighted the sensitivity of the crack initiation location to the contact angle and questioned its use.^{7,8} The debate is very much open and a myriad of papers have been published trying to shed light on the validity regimes of the Brazilian test using theoretical, numerical and experimental tools. Recent examples include the work of Alvarez-Fernandez and co-workers⁹, who investigated, experimentally and analytically, the influence of the contact angle in the stress distribution and the failure load in slate. They reported that contact angles in the range $23 - 32^\circ$ were the most suitable to achieve crack initiation near the disk centre. Markides and Kourkoulis¹⁰ used analytical methods to evaluate the sensitivity of the stress state to the jaw's curvature, delimiting the conditions where Eq. (2) is applicable. Gutierrez-Moizant et al.¹¹ conducted Brazilian tests in concrete with various contact angles and recommended using a loading arc of 20° . Bouali and Bouassida¹² investigated the role of the contact angle for both concrete and mortar, concluding that 20° was the most suitable contact angle for concrete while 10° was recommended for mortar. Garcia-Fernandez et al.¹³ conducted Brazilian tests in PMMA samples, which enabled them to visualise the crack initiation process and demonstrated the important role of the contact angle. Zhao and co-workers¹⁴ used acoustic emission to investigate the role of the experiment setup on the crack nucleation event. Aliabadian et al.¹⁵ showed, using Digital Image Correlation (DIC), that the location of crack nucleation was sensitive to the contact angle and estimated a value of $\alpha = 25^\circ$ as the most appropriate one for sandstone. Alternative testing configurations have also been proposed (see, e.g., Refs. 16, 17 and Refs. therein). The aforementioned studies provide material-specific estimations of test geometry (contact angles) that result in a stress state where the maximum tensile stress is attained at the centre of the disk. This can be achieved by using a sufficiently large jaw radius (sufficiently small contact angle). However, small contact angles result in high contact stresses that cause premature cracking near the loading region.¹⁸ Thus, finding a suitable testing configuration involves striking a balance between ensuring that the contact angle is both: (i) small enough such that the maximum tensile stress is attained at the centre and Eq. (2) is satisfied, and (ii) large enough such that cracking does not

occur in the compressive region beneath the jaw. This is not straightforward as it depends on a number of testing and material parameters and even today technical standards differ in their recommendations (see, e.g., Refs. 4, 19). There is a need for a generalised approach that will enable mapping the regimes of validity of the Brazilian test for arbitrary choices of material and test configuration.

In this work, we use the generalised Griffith criterion^{7,20} to gain insight into the location of crack initiation in the Brazilian test. By considering the entire failure envelope, we ensure that not only is the maximum tensile stress attained at the centre of the sample at the moment of failure but also that this crack nucleation event is not preceded by cracking elsewhere in the sample. Finite element calculations are conducted to build maps that enable assessing the experiment viability for any material and test geometry. First, we analyse the stress state at the disk centre as a function of the load and quantify the error associated with Hondros's solutions, Eqs. (1) and (2), for relevant material properties and testing configurations. Second, we map the conditions that lead to crack nucleation at the disk centre and thus to a valid test. Calculations span the main classes of rocks and assess the suitability of current testing standards. We find that the range of conditions where a Brazilian test is valid is much narrower than previously thought. A protocol is presented to ensure that the experiment leads to a valid estimate of the material tensile strength. This is exemplified with specific case studies and facilitated by providing a MATLAB App that takes as input the test data and provides as output the validity of the experiment and the magnitude of the tensile strength.

2. Generalised Griffith criterion for crack initiation

Griffith²⁰ studied the fracture of brittle materials under compressive loads by assuming that the rupture process was driven by local flaws within the material. As shown in Fig. 2, local tensile stresses will develop near existing flaws when these are oriented at an angle relative to the principal directions of the applied stress. Denoting the major and minor principal stresses as σ_1 and σ_3 , respectively, Griffith's²⁰ two-part criterion for the onset of fracture is given as follows,^d

$$\begin{cases} \sigma_1 = \sigma_t & \text{if } 3\sigma_1 + \sigma_3 \geq 0 \\ (\sigma_1 - \sigma_3)^2 = -8\sigma_t(\sigma_1 + \sigma_3) & \text{if } 3\sigma_1 + \sigma_3 < 0 \end{cases} \quad (3)$$

^d A detailed derivation can be found in Ref. 21.

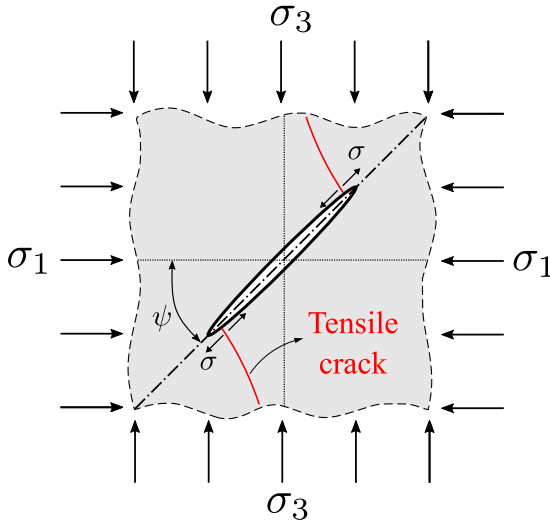


Fig. 2. Local stress state in a Griffith micro-crack, with ψ denoting the crack inclination angle. When the local tensile stresses reach the material tensile strength σ_t , wing cracks nucleate near the edges of the original micro-crack.

with the initial crack orientation being respectively given by the angles:

$$\begin{cases} \psi = \pi/2 & \text{if } 3\sigma_1 + \sigma_3 \geq 0 \\ \psi = \frac{1}{2} \cos^{-1} \left(\frac{\sigma_1 - \sigma_3}{2(\sigma_1 + \sigma_3)} \right) & \text{if } 3\sigma_1 + \sigma_3 < 0 \end{cases} \quad (4)$$

Two aspects must be emphasised. First, it is observed that for a regime where σ_1 is tensile and σ_3 is compressive with an absolute value lower than three times σ_1 , conditions of purely tensile failure take place, with cracks parallel to the original flaw²². Second, the criterion indicates that the material compressive strength σ_c is eight times its tensile strength as Eq. (3)b gives $\sigma_3 = \sigma_c = -8\sigma_t$ under uniaxial compression ($\sigma_1 = 0$). While this is of the right order of magnitude, it limits the application of the criterion to materials with a compressive-to-tensile strength ratio of 8. To overcome this and generalise Griffith's criterion, Fairhurst⁷ proposed an extension to allow for arbitrary compression-to-tensile strength ratios. This is achieved by defining a parabolic Mohr envelope that encloses the uniaxial tensile and compressive strength circles, with the former being touched at its vertex and the latter being tangent to the envelope — see Fig. 3a. Accordingly, defining n as the compressive-to-tensile strength ratio ($n = -\sigma_c/\sigma_t$), the relation describing the compressive strength circle is given by,

$$\left(\sigma + \frac{n\sigma_t}{2} \right)^2 + \tau^2 = \left(\frac{n\sigma_t}{2} \right)^2 \quad (5)$$

with σ and τ respectively denoting the normal and shear stresses.

In terms of the principal stress space, the generalised Griffith criterion reads:

$$\begin{cases} \sigma_1 = \sigma_t & \text{if } m(m-2)\sigma_1 + \sigma_3 \geq 0 \\ \sigma_3 = \sigma_1 - (1-m)^2\sigma_t + 2(1-m)\sqrt{\sigma_t(\sigma_t - \sigma_1)} & \text{if } m(m-2)\sigma_1 + \sigma_3 < 0 \end{cases} \quad (6)$$

where m is a material parameter defined as $m = \sqrt{n+1}$. The failure envelope is shown graphically in Fig. 3b. The generalised Griffith criterion particularises to the original Griffith criterion (3) for $n = 8$ and otherwise extends it to arbitrary tensile and compressive material strengths. It is worth noting that the adoption of the generalised Griffith criterion necessarily implies that the Brazilian test is, generally, not a

suitable experiment for measuring the tensile strength of materials with $n < 8$; see Eq. ((6)a) and Fig. 3b and consider the fact that $\sigma_3 \approx -3\sigma_1$ at the disk centre for zero or small contact angles.²¹

3. The application of Griffith's criterion to the Brazilian test

During the Brazilian split test, the material points in the disk undergo a stress state that is characterised by two domains in the principal stress state — see Fig. 4. In some regions, such as in the vicinity of the jaws, material points exhibit compressive major and minor principal stresses ($\sigma_1 < 0$ & $\sigma_3 < 0$). However, near the centre of the disk, the stress state is characterised by a maximum principal stress in tension ($\sigma_1 > 0$) and a minimum principal stress in compression ($\sigma_3 < 0$).

As discussed in Section 1, the controversy surrounding the Brazilian test is related to the crack initiation location. For the experiment to provide a valid estimate of the material tensile strength, the onset of cracking must take place at the centre of the disk and the relation between the critical load and σ_1 at the disk centre must be known. One can use the failure envelope of the generalised Griffith criterion (Fig. 3) to analyse the stress state in the disk and map the conditions of validity. This is shown in a schematic manner in Fig. 5, where a cloud of points is used to represent the potential stress states in a discrete number of material points distributed within the disk, $(\sigma_1, \sigma_3)_{(x,y)}$. Two scenarios can essentially occur. On the one hand, Fig. 5a, the test is invalid if the first material point reaching the failure envelope is not located in the centre of the disk. This is, for example, what happens when cracking is observed close to the loading jaws. On the other hand, Fig. 5b, if the failure envelope is reached first by the material point located at the disk centre ($x = 0, y = 0$), then a valid estimate of the tensile strength is obtained: $\sigma_t = (\sigma_1)_{(0,0)}$.

For a given applied load, test geometry and elastic properties of jaws and disk, the validity of the test will be determined by the failure envelope (i.e., the magnitude of σ_c and σ_t). Fig. 5c shows a scenario where one of the conditions of validity of the Brazilian test has been met: the centre of the disk (green dot) is in a stress state where $(\sigma_1)_{(0,0)} = \sigma_t$. However, the test is still not valid if the ratio σ_c/σ_t is sufficiently low — several material points are above the envelope, implying that failure has occurred elsewhere at a smaller load. This scenario is illustrated with a red dotted curve in Fig. 5c. If the ratio σ_c/σ_t is sufficiently large (green dashed curve), then the only point in contact with the envelope is the centre one, and the experiment is valid.

The limiting case is that where the failure envelope is met at two or more points at the same time, one of which is located at the disk centre. This is illustrated in Fig. 5c with an orange dash-dotted line and provides the threshold of admissible σ_c/σ_t ratios for a Brazilian test to be valid. Thus, for a given load, geometry and material parameters, one can use numerical analysis to estimate the stress state at any point in the disk $(\sigma_1, \sigma_3)_{(x,y)}$ and utilise the generalised Griffith criterion to determine the compressive strength associated with a failure envelope passing through that point; i.e., re-arranging Eq. (6)b:

$$(\sigma_c)_{(x,y)} = -\sigma_t \left[\frac{\left(\sigma_t - \sqrt{\sigma_t(\sigma_t - (\sigma_1)_{(x,y)})} + \sqrt{\sigma_t(\sigma_t - (\sigma_3)_{(x,y)})} \right)^2}{\sigma_t^2} - 1 \right] \quad (7)$$

For the failure condition to be first met at the disk centre, the maximum value of $(\sigma_c)_{(x,y)}$ among all material points in the disk, as estimated via Eq. (7), must be equal or smaller than the real material compressive strength σ_c . Hence, since σ_c is a known material property that can be measured independently, one can combine numerical analysis and the generalised Griffith's criterion to map the conditions that lead to failure initiation from the centre of the disk. In this way, the two validity conditions of the Brazilian test – cracking initiating at the centre (0,0) and $(\sigma_1)_{(0,0)} = \sigma_t$ – can be incorporated in the analysis, as shown below.

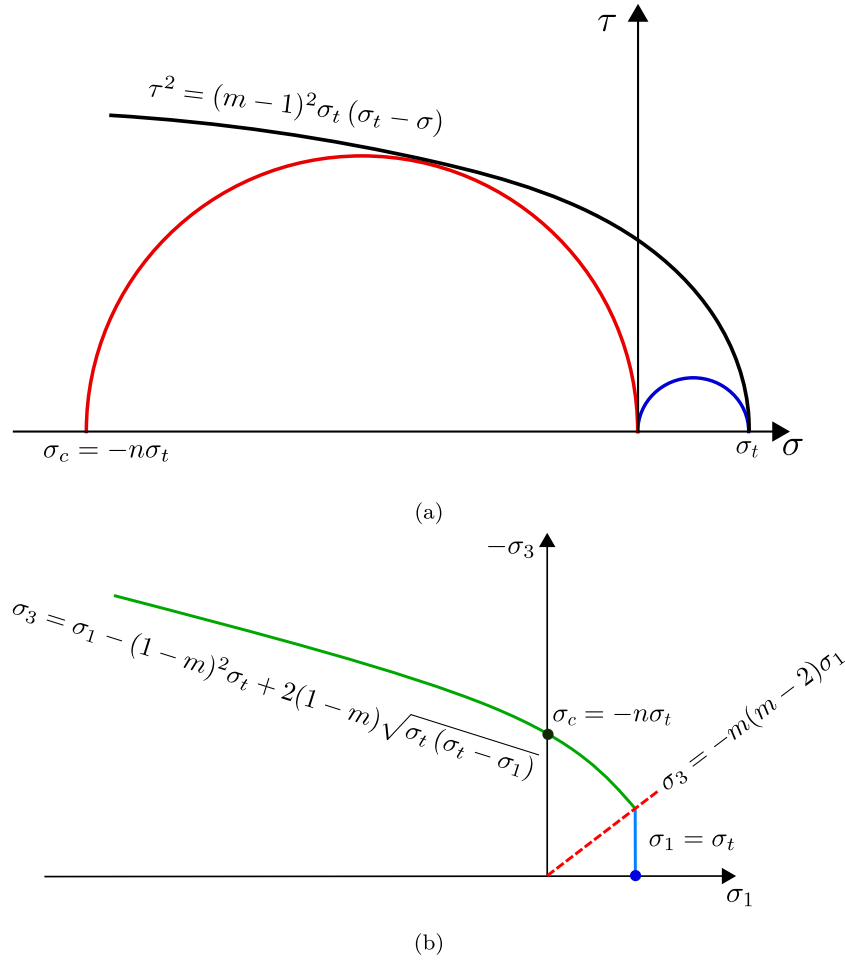


Fig. 3. Generalised Griffith criterion. Mohr diagram showing the generalised parabolic failure envelope in terms of: (a) normal σ and shear τ stresses, and (b) minor σ_3 and major σ_1 principal stresses. Here, $n = -\sigma_c/\sigma_t$ and $m = \sqrt{n+1}$.

4. Analysis

We proceed to combine finite element analysis and the generalised Griffith criterion to map the regimes of validity of the Brazilian test.

4.1. Preliminaries

The location of crack initiation in the Brazilian test is a function of 2 geometrical and 6 material parameters: the jaw radius (R_j), the disk radius (R_d), the elastic properties of the disk (E_d , ν_d) and jaws (E_j , ν_j), and the tensile (σ_t) and compressive (σ_c) strengths of the material being tested. Assuming that cracking initiates along the vertical middle axis of the disk, the crack initiation location can be fully characterised by a variable Y , equal to 0 at the centre and to R_d at the edge. Then, dimensional analysis dictates that the solution is a function of the following non-dimensional sets:

$$\frac{Y}{R_d} = F\left(\frac{R_j}{R_d}, \frac{E_j}{E_d}, \nu_j, \nu_d, \frac{\sigma_c}{E_d}, \frac{\sigma_t}{E_d}\right). \quad (8)$$

Further assuming that crack nucleation takes place at the centre of the disk ($Y/R_d = 0$), as required for the test to be valid, then Eq. (8) can be re-arranged to:

$$\frac{\sigma_c}{\sigma_t} = G\left(\frac{R_j}{R_d}, \frac{E_j}{E_d}, \nu_j, \nu_d, \frac{\sigma_c}{E_d}\right). \quad (9)$$

Thus, conducting calculations over relevant ranges of the five non-dimensional sets in Eq. (9) will enable mapping the conditions that lead to cracking at the disk centre.

We use the GRANTA Material library²³ to define a suitable range of material properties. The Young's modulus, Poisson's ratio, tensile strength and compressive strength of the most widely used rock-like materials are shown in Figs. 6a–6c. To conduct a comprehensive analysis, we vary the Young's modulus of the disk from 5 to 150 GPa. Also, Poisson's ratio is varied within the range 0.1 to 0.4. The jaws are typically made of steel and thus the following elastic properties are assumed: $E_j = 210$ GPa and $\nu = 0.3$. Given that E_j and ν_j are fixed (and known), the dimensional analysis conducted above suggests that the two critical non-dimensional sets are σ_c/σ_t and σ_c/E_d . Thus, we proceed to plot their relationship for a wide range of materials in Fig. 6d. It can be observed that relevant ranges of σ_c/σ_t and σ_c/E_d are approximately 2–30 and 0.0001–0.01, respectively.

To determine the stress state within the disk we conduct finite element analysis of the contact between the jaws and the sample and the subsequent material deformation. The commercial finite element package ABAQUS is used. Only one quarter of the test is simulated, taking advantage of symmetry. The radius of the disk equals $R_d = 10$ mm while the jaw radius is varied from $R_j = 11$ mm to the case of a flat jaw geometry ($R_j \rightarrow \infty$). Quadratic quadrilateral finite elements with full integration are used to discretise the disk and the jaw. Plane strain conditions are assumed. After a sensitivity analysis, a total of 28,241 elements are used to discretise the disk and between 4102 and 4459 elements are used for the jaw. The mesh is particularly fine in the disk and in the regions of the jaw that are in contact with the disk. A uniform negative vertical displacement is applied at the top of the jaw and the resulting reaction force is measured. The contact behaviour is modelled as follows. For the normal behaviour, we

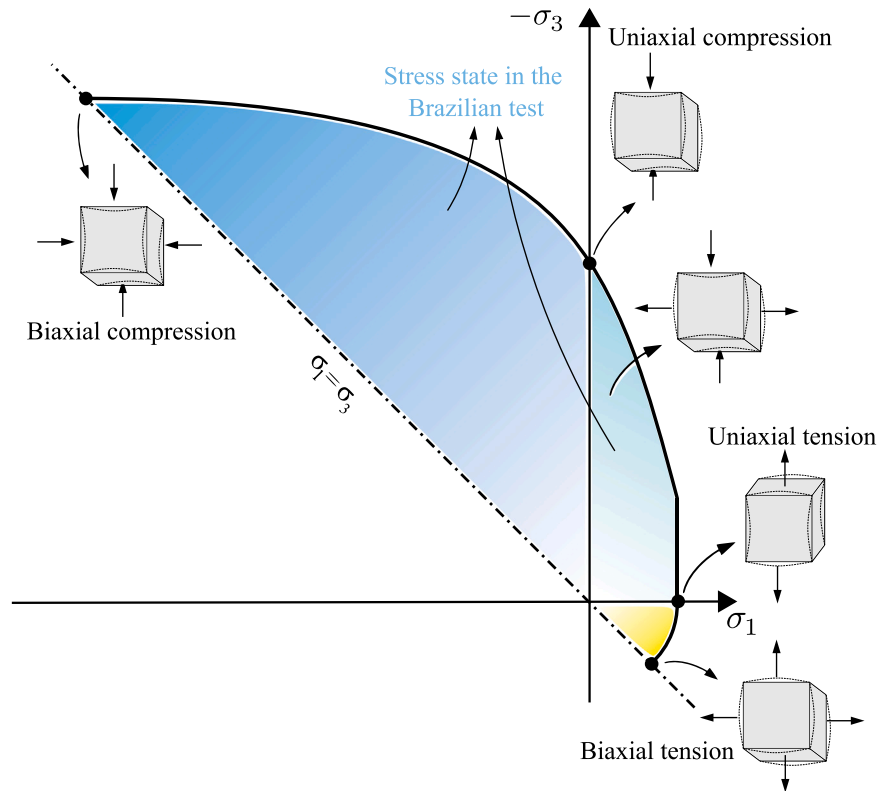


Fig. 4. Stress states and typical failure envelope for rock-like materials, emphasising the two regimes relevant to the Brazilian test. The stress states are shown in the principle stress diagram, with tensile stresses being positive and σ_1 and σ_3 respectively denoting the major and minor principal stresses.

consider surface-to-surface hard contact, where Lagrangian multipliers are used to ensure that the contact pressure and the contact constraint minimise overclosure. For the tangential behaviour, frictionless contact is generally assumed although the role of friction is also investigated (see Section 4.3.4), revealing a negligible influence.

4.2. Mapping the stress state at the disk centre

We shall start by quantifying the relationship between the load P and the stress state at the centre of the disk under a wide range of conditions. The goal is to map the scenarios where Eqs. (1) and (2) are valid. We shall start by assessing the validity of Eq. (2), an intrinsic assumption in the standards. The finite element results obtained are shown in Fig. 7 in terms of the stress state at the centre of the disk ($x = 0, y = 0$) versus the load for a wide range of E_j/E_d values and selected choices of jaw radius, as given by the ratio R_j/R_d . In terms of test geometry, three scenarios are considered: $R_j/R_d = 1.1$, $R_j/R_d = 1.5$ (as in the ISRM standard) and flat jaws (one of the configurations recommended by the ASTM standard). The limits of the x -axis are chosen so as to encompass a wide range of realistic contact angles; the upper limit ($P/(\pi R_d t) = 0.0003 E_j$) corresponds to a tensile strength of roughly 60 MPa if a steel jaw ($E_j = 210$ GPa) is considered in Eq. (2), which is sufficiently high to cover the vast majority of rock-like materials.

The results reveal that Eq. (2) is only valid for low load magnitudes and small E_j/E_d ratios. The error is particularly significant for low R_j/R_d values — note the y axis limits in Fig. 7a. But even for the case of flat jaws, as recommended by the ASTM standard, $(\sigma_1)_{(0,0)}/(P/\pi R_d t)$ is only equal to 1 for low contact angles (low P) and small Young's modulus mismatch. Consider for example a sandstone with $E_d = 20$ GPa ($E_j/E_d = 10.5$) and tensile strength $\sigma_t = 20$ MPa ($P/(\pi R_d t) \approx 0.0001 E_j$), see Fig. 6; in all cases Eq. (2) is not fulfilled, with the errors being of roughly 5%, 2% and 0.5% for, respectively, the cases of $R_j/R_d = 1.1$, $R_j/R_d = 1.5$ (as suggested by ISRM) and flat jaws (as suggested by

the ASTM standard). The maximum errors observed for these three configurations, relevant to materials with high tensile strength and low stiffness, are respectively 36%, 13% and 5%. However, these maps enable a precise determination of the stress state in the centre of the disk and, accordingly, of the material tensile strength σ_t . One can use them to assess if the error intrinsic to the adoption of the point load equation is admissible, or directly as a replacement to Eq. (2), as these maps enable determining the precise value of $\sigma_1 (= \sigma_t)$ at the disk centre as a function of the material properties, test geometry and critical load.

The results obtained for a wide range of jaw radii are given in Fig. 8. Maps are provided as a function of the normalised load, using the Young's modulus of the rock as normalising parameter. Two figures are shown, corresponding to the lower and upper bounds of the elastic modulus; $E_j/E_d = 42$ ($E_d \approx 5$ GPa, Fig. 8a) and $E_j/E_d = 1.4$ ($E_d \approx 150$ GPa, Fig. 8b). Maps for other scenarios are provided in the Supplementary Material, so that experimentalists can accurately determine the stress state at the disk centre for arbitrary materials and test conditions. See also the Matlab App described in Appendix A. In agreement with expectations and with the results shown in Fig. 7, stiffer materials bring the stress state close to that fulfilling Eq. (2). Also, the error is relatively small when large jaw radii are used, with the limiting case being given by the flat jaws recommended by ASTM.¹⁹

Let us assume that the contact angle can be experimentally determined and assess the accuracy of Hondros's analytical solution for $\alpha > 0$, Eq. (1). The finite element prediction of maximum principal stress at the disk centre is shown in Fig. 9 normalised by Hondros's analytical solution for a uniformly distributed load. Results are shown for the lower and upper bounds of the elastic modulus considered above, and as a function of the jaw radius. Differences are overall small, as could be expected from Saint-Venant's principle. However, the assumption of a uniform pressure, intrinsic to Hondros's solution, leads to errors above 3% for softer rocks and curved jaw configurations such as that of the ISRM standard. As in Figs. 7 and 8, the error becomes negligible for rocks on upper end of the stiffness spectrum and for jaws with

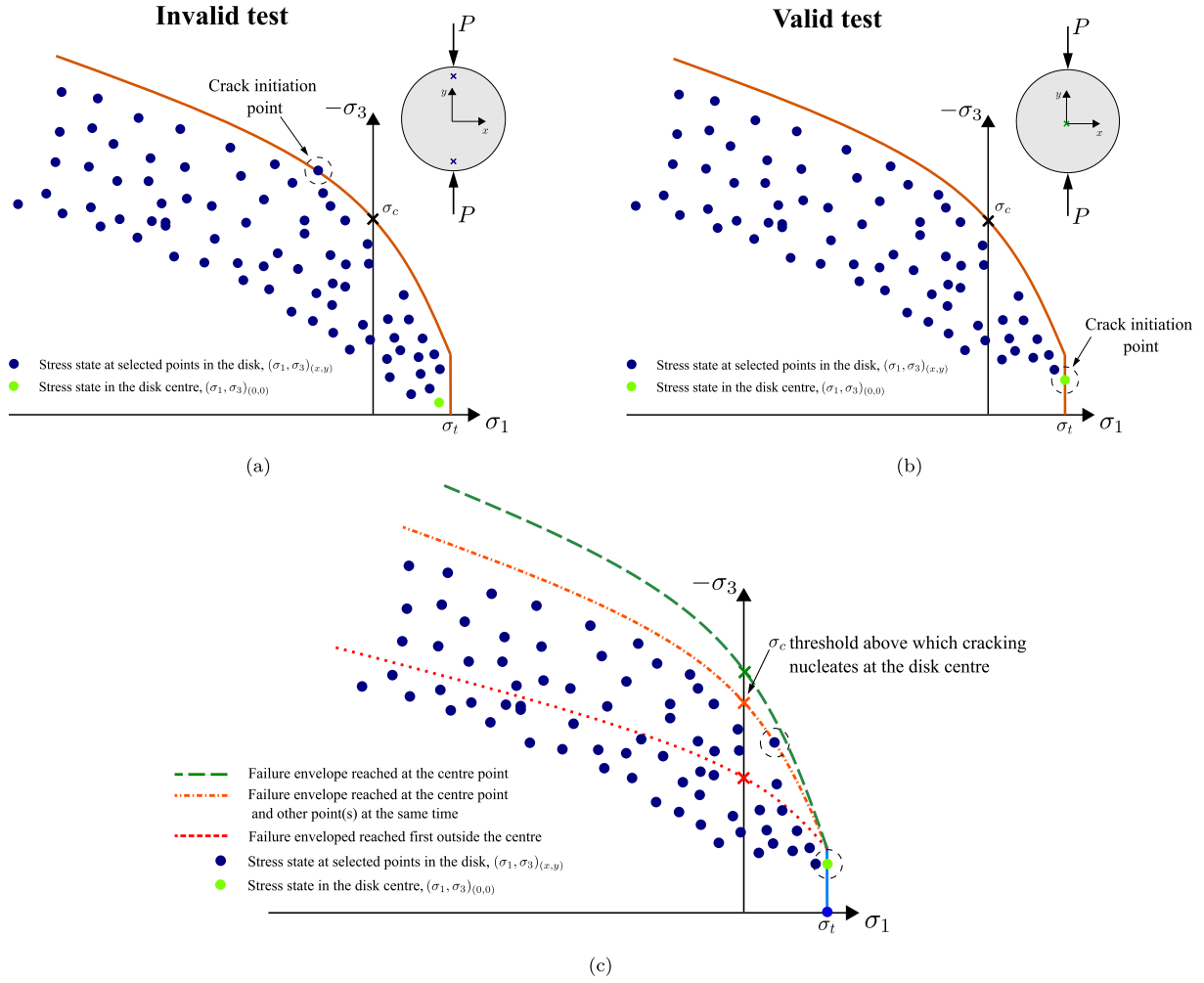


Fig. 5. Stress state at a discrete number of material points within the Brazilian disk and failure envelopes based on the generalised Griffith criterion. (a) Conditions leading to an invalid test; failure is attained outside from the disk centre. (b) Conditions leading to a valid test; $\sigma_1 = \sigma_t$ at the centre of the sample (0,0). (c) Validity of the test as a function of the failure envelope (σ_c , σ_t) for a given stress state associated with a load P . A green dot is used to denote the stress state at the disk centre (0,0).

large radius. Notwithstanding, as discussed below, the use of a large jaw radius favours the nucleation of cracking far from the disk centre, making the test invalid.

4.3. Mapping the conditions that lead to cracking at the disk centre

Low contact angles lead to stress states that are close to the Hondros equations. However, this is not sufficient for the test to be valid as cracking can nucleate outside of the disk centre, as it is often reported when flat or large-radius jaws are used (see, e.g., Refs. 12, 24, 25). While the maps presented in Section 4.2 provide a relationship between the critical load and the tensile strength (even if Eq. (2) is not met), this is only meaningful if the critical load is associated with the initiation of cracks at the disk centre and not elsewhere. To determine the location of crack nucleation, we combine the generalised Griffith failure envelope and finite element analysis (see Section 3). To achieve this, we start by assuming that cracking initiates at the disk centre, where $\sigma_1 = \sigma_t$, and assess that assumption by comparing the compressive-to-tensile strength ratio resulting from the test with the admissible range of σ_c/σ_t ratios. If the latter is greater than the former, then cracking initiates outside of the disk centre and the test is invalid. Specifically, for each combination of material and test parameters, the process is as follows. Firstly, a finite element analysis is conducted to estimate the principal stresses (σ_1 , σ_3) at each integration point for a wide range of load increments. Secondly, Eq. (7) is used to compute

the minimum admissible σ_c (i.e., the maximum σ_c among all material points). Finally, from the threshold σ_c and the assumption $(\sigma_1)_{(0,0)} = \sigma_t$, a data point is established relating the material and test parameters to the threshold of admissible σ_c/σ_t values. Each map, such as Fig. 10a, is built using approximately 20,000 of these data points and interpolating in-between. The process is automated by means of Python and MATLAB scripts.²⁶

4.3.1. The influence of the jaw radius

We start by mapping the influence of the jaw radius on the validity of the Brazilian test. Fig. 10 shows, following the procedure described above, the relation between the jaw radius (as given by R_j/R_d), the non-dimensional set σ_c/E_d and the minimum acceptable compressive-to-tensile strength ratio. Maps are provided for two limit cases of disk elastic properties: $E_j/E_d = 42$ (i.e., $E_d \approx 5$ GPa) and $E_j/E_d = 1.4$ (i.e., $E_d \approx 150$ GPa), with the majority of rock-like materials expected to fall between these two cases. By comparing Figs. 10a and 10b, it can be seen that while E_j/E_d influences the results, the role appears to be of secondary nature relative to the influence of the jaw radius.

The results reveal the following trends. First, for a given jaw radius, the range of admissible σ_c/σ_t ratios increases with increasing σ_c/E_d , as valid tests (centre cracking) are those above the σ_c/σ_t threshold. When the compressive strength increases, the likelihood of cracking nucleating outside of the disk centre decreases. For example, consider the specific case $E_j/E_d = 42$ and $R_j/R_d = 1.5$. When $\sigma_c/E_d = 0.002$, the

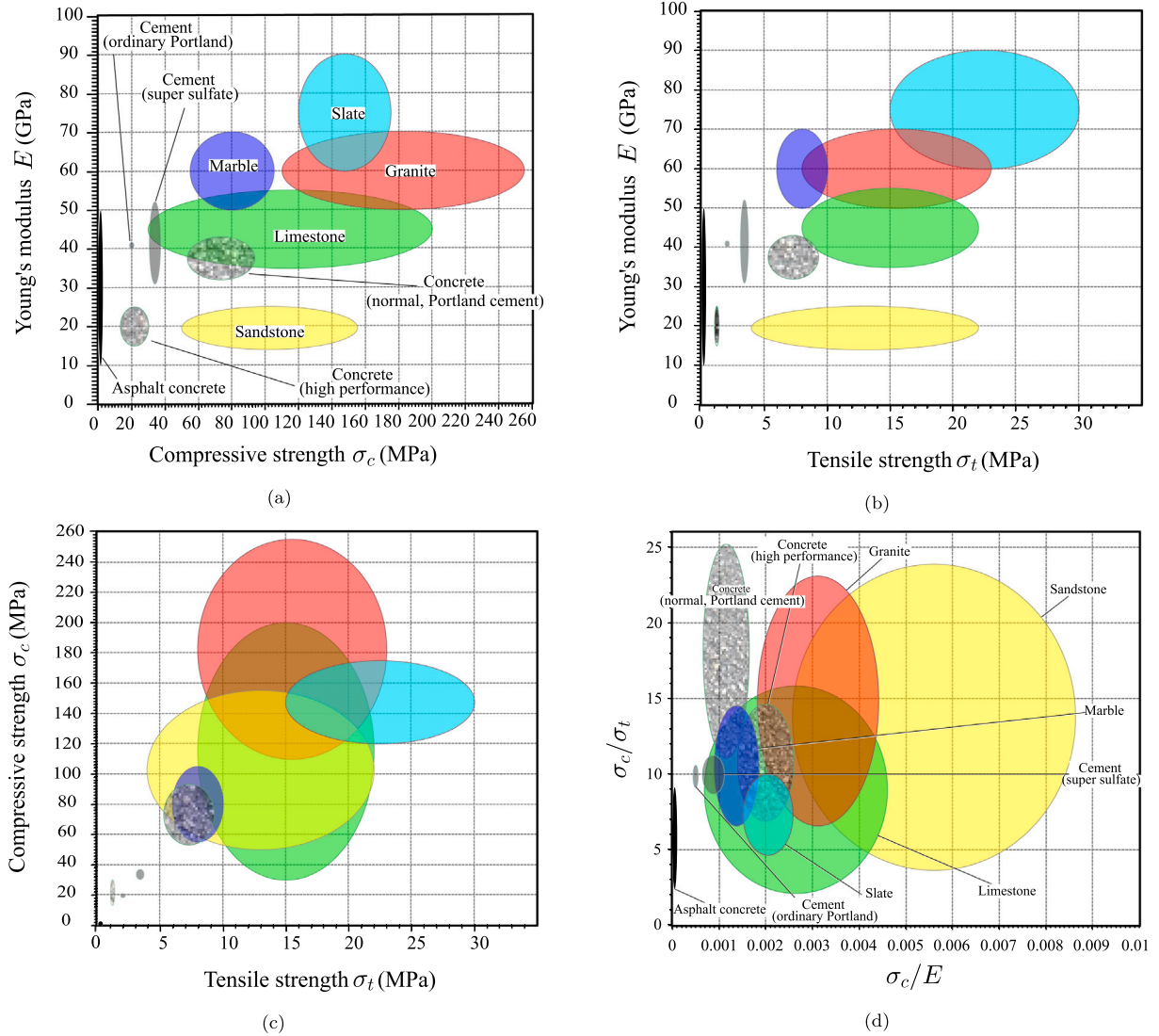


Fig. 6. Material property range of rock-like materials. Ashby charts showing the relations between (a) Young's modulus (E) and compressive strength (σ_c), (b) Young's modulus (E) and tensile strength (σ_t), (c) compressive (σ_c) and tensile (σ_t) strengths, and (d) ratio of compressive-to-tensile strength (σ_c/σ_t) and ratio of compressive strength to elasticity modulus σ_c/E . The data is taken from the GRANTA Material library²³ for granite, slate, marble, sandstone, limestone, concrete, cement and asphalt. The typical ranges for the Poisson's ratio of these materials are: granite $\nu = 0.15 - 0.26$, slate $\nu = 0.22 - 0.3$, marble $\nu = 0.14 - 0.22$, sandstone $\nu = 0.22 - 0.29$, limestone $\nu = 0.2 - 0.26$, concrete $\nu = 0.1 - 0.2$, cement $\nu = 0.2 - 0.24$, and asphalt $\nu = 0.35 - 0.36$.

region of validity is $\sigma_c/\sigma_t > 28$, whereas when $\sigma_c/E_d = 0.01$, the ratio σ_c/σ_t needs only to exceed 11. Also, lower E_d values result in larger contact angles and thus less chances of cracking occurring nearby the loading jaws. This is also observed by comparing Figs. 10a and 10b; the stiffer the sample the more likely that cracking will occur in the compressive regions. Importantly, the results provide σ_c/σ_t thresholds below which it is not possible to obtain a valid Brazilian test. Thus, it is not possible to obtain a valid result if $\sigma_c/\sigma_t < 7$, independently of the jaw radius. For σ_c/E_d ratios as high as 0.01, the ISRM ($R_j/R_d = 1.5$) configuration provides thresholds of σ_c/σ_t equal to 11 (Fig. 10a) and 8 (Fig. 10b). While the ASTM (flat jaws) configuration gives σ_c/σ_t thresholds of 20 (Fig. 10a) and 14 (Fig. 10b). Hence, as it can be seen in Fig. 6(d), conducting Brazilian tests in agreement with the ISRM and (particularly) ASTM guidelines will lead to invalid results for a range of rocky materials, independently of the jaw radius.

4.3.2. The influence of Young's modulus

We proceed to report the effect of the Young's modulus of the sample (E_d) for selected testing geometries. Specifically, results are shown for a small jaw radius ($R_j/R_d = 1.1$) and the ISRM ($R_j/R_d = 1.5$)

and ASTM (flat jaws) recommended configurations. The maps obtained are presented in Fig. 11.

Several observations can be drawn. First, the flatter the jaws the higher the sensitivity to the elastic stiffness of the sample. The map is wider and more significant differences can be observed between the admissible limits for a given σ_c/E_d value. A smaller range of admissible σ_c/σ_t ratios (i.e., lower threshold values) is predicted with increasing jaw radius. This is consistent with expectations in terms of contact angles; high contact angles can readily be achieved with curved jaws while flat or large radius jaws can only do so if the disk is soft. Second, the figure emphasises the limitations of current standardised procedures. As shown in Fig. 6(d), many materials lie within the region delimited by 0.001–0.004 σ_c/E_d and 5–15 σ_c/σ_t . However, the maps obtained for the ISRM and ASTM standards fall above this region, implying that the tests will necessarily result in estimates below the admissible σ_c/σ_t threshold and thus cracking is predicted to occur in the compressive region, rather than in the disk centre.

4.3.3. The influence of Poisson's ratio

The role of the disk's Poisson's ratio is examined in Fig. 12. Two limit values are considered, $\nu_d = 0.1$ and $\nu_d = 0.4$, and results are

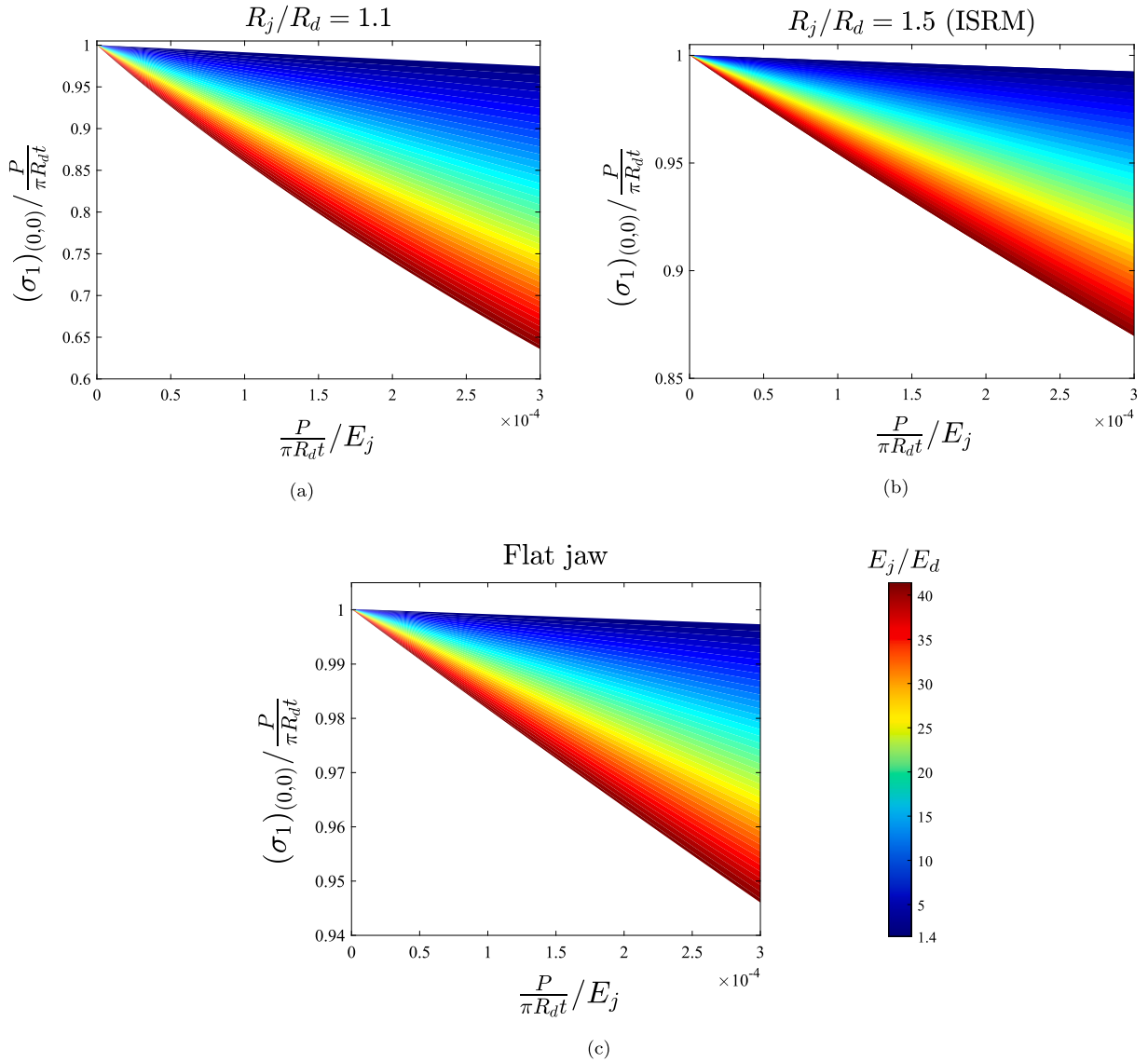


Fig. 7. Maps to quantify the stress state at the disk centre as a function of the material properties, test geometry and critical load. Normalised major principal stress versus dimensionless load for a wide range of E_j/E_d values and the following test geometries: (a) $R_j/R_d = 1.1$, (b) $R_j/R_d = 1.5$ (as recommended by ISRM), and (c) flat jaws (as recommended by ASTM). Poisson's ratio in the disk is taken to be $\nu_d = 0.2$.

obtained for limit cases of E_j/E_d and R_j/R_d so as to span all scenarios. Overall, Poisson's ratio seems to play a very secondary role. The effect is negligible for low jaw radii ($R_j/R_d = 1.1$) and this appears to be insensitive to the elastic modulus mismatch (E_j/E_d). Some differences are observed for jaws with a large radius, with smaller Poisson's ratios further reducing the range of admissible compressive-to-tensile strength ratios. This implies that the appropriate value of Poisson's ratio must be used when assessing the validity of the Brazilian test in a configuration with flat or large-radius jaws, as in the ASTM standard.¹⁹

4.3.4. The influence of friction

To investigate the role of friction, simulations are conducted with a friction coefficient of $\mu = 0.8$, an upper bound with respect to the values that may be expected for rock/metal interfaces. A penalty method is used to incorporate friction into the model. As in the Poisson's ratio study, we consider limit values of E_j/E_d and R_j/R_d , to span all relevant conditions. The results are shown in Fig. 13 for a Poisson's ratio of $\nu_d = 0.1$; consistent with the observations above, other values of the disk's Poisson's ratio led to identical conclusions. As it can be observed, no noticeable differences are seen between the simulations

with and without friction. This also holds for other values of the friction coefficient (results not shown) and is in agreement with the secondary role of friction reported in the literature.^{27–30} While friction is known to influence the stress state of material points near the jaws,³¹ these points appear to play a secondary role in our analysis of the validity of the Brazilian test.

4.4. Representative case studies

Let us now showcase the importance of the maps presented above by particularising them to the study of common rock materials. Fig. 14 shows the results obtained for granite, sandstone, limestone and marble. To build the maps, a Poisson's ratio of $\nu_d = 0.2$ is adopted in all cases, while the Young's modulus equals $E_d = 60$ GPa ($E_j/E_d = 3.5$) for granite, $E_d = 20$ GPa ($E_j/E_d = 10.5$) for sandstone, $E_d = 50$ GPa ($E_j/E_d = 4.2$) for limestone, and $E_d = 60$ GPa ($E_j/E_d = 3.5$) for marble. The space that these materials occupy in a compressive-to-tensile strength ratio versus σ_c/E_d plot is shown by means of ellipses, based on the material properties available in the GRANTA Material library²³ (see Fig. 6(d)). As before, estimates of the admissible σ_c/σ_t

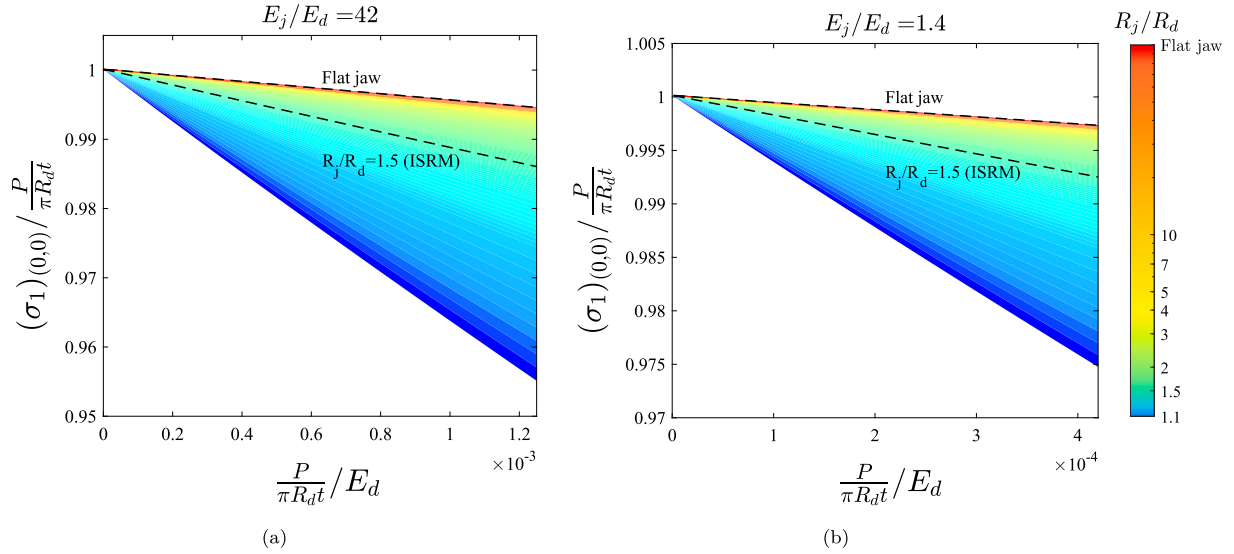


Fig. 8. Maps to quantify the stress state at the disk centre as a function of the material properties, test geometry and critical load. Normalised major principal stress versus dimensionless load for a wide range of R_j/R_d values and the following bounds of the elastic stiffness: (a) $E_j/E_d = 42$, and (b) $E_j/E_d = 1.4$. Poisson's ratio in the disk is taken to be $\nu_d = 0.2$.

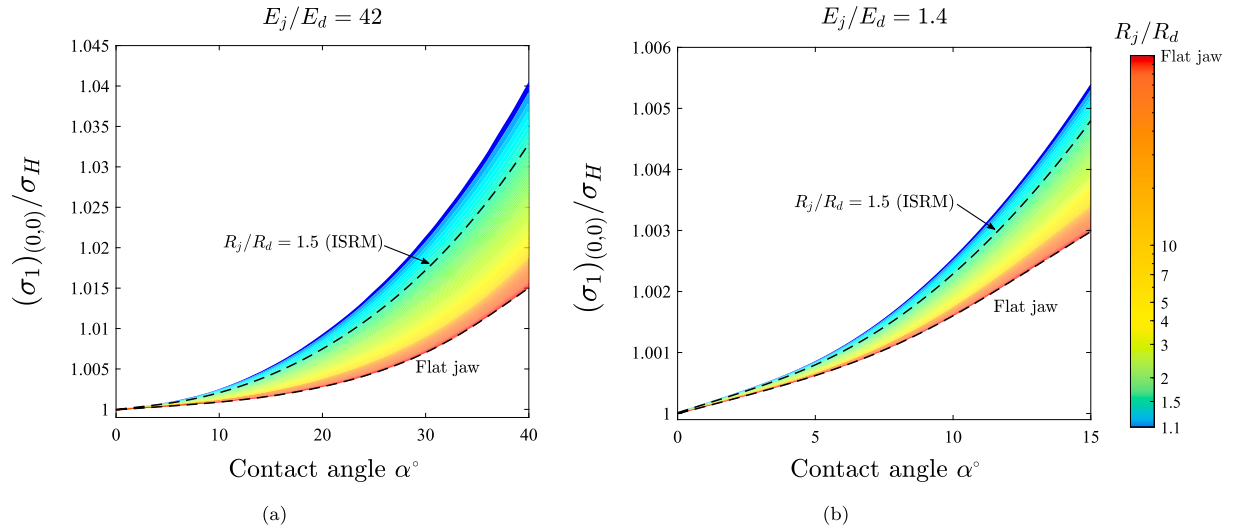


Fig. 9. Maps to evaluate the accuracy of Hondros's analytical solution for a uniformly distributed load, Eq. (1). The finite element predictions of the maximum principal stress at the disk centre are normalised by Hondros's stress solution, denoted as σ_H . The results are obtained for a wide range of R_j/R_d values and the following bounds of the elastic stiffness: (a) $E_j/E_d = 42$, and (b) $E_j/E_d = 1.4$.

ratios are provided for jaw radii varying from $R_j/R_d = 1.1$ to the flat jaws recommended by the ASTM standard.¹⁹

Consider first the case of granite, Fig. 14a. Flaw radii from $R_j/R_d = 1.1$ to $R_j/R_d = 2.2$ can be used to obtain valid estimates for granite materials within the upper estimates of compressive-to-tensile strength ratios. This includes the ISRM configuration ($R_j/R_d = 1.5$), which appears to be suited for some classes of granite. The number of suitable testing configurations improves for sandstone, see Fig. 14b. Types of sandstone can be adequately tested with jaw radius up to $R_j/R_d = 7$ but the use of flat jaws would lead to an invalid result and no testing configuration is suitable for sandstones with low σ_c/σ_t ratios. In the case of limestone, see Fig. 14c, only jaw radii from $R_j/R_d = 1.1$ to $R_j/R_d = 1.3$ can be used and these cover only those limestones with high compressive strength. In this case, it is not possible to get a valid estimate of σ_t with the ISRM testing configuration for any type of limestone. Finally, the results obtained for marble (Fig. 14d) show that

only a small class of marbles can be adequately characterised with the Brazilian test, and this requires using the smallest jaw radius considered ($R_j/R_d = 1.1$). Again, as in the case of limestone, it does not appear to be possible to measure the tensile strength of any class of marble using the Brazilian test configuration suggested by the ISRM. Remarkably, the flat jaws recommended by the ASTM standard are shown to be generally unsuited to provide a valid estimate of the tensile strength, across the wide range of granites, sandstones, limestones and marbles considered.

The maps presented can be used by experimentalists to assess the validity of the their testing configuration, as described below. To facilitate this, we provide as Supplementary Material admissible σ_c/σ_t maps for relevant ranges of material properties and testing parameters. Moreover, as described in Appendix A, a MATLAB App is provided that includes a convenient graphical user interface to readily confirm the validity of the test, based on the criteria and analyses conducted here.

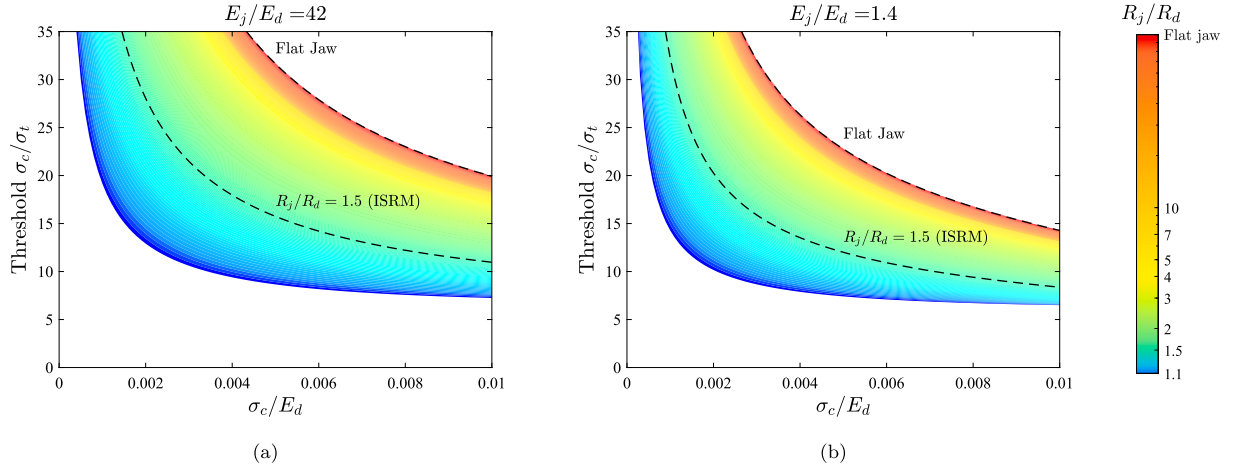


Fig. 10. Maps to assess if cracking nucleates at the centre. Influence of the jaw radius on the minimum acceptable ratio of compressive-to-tensile strength for (a) $E_j/E_d = 42$ and (b) $E_j/E_d = 1.4$. The disk's Poisson's ratio equals $\nu_d = 0.2$. Dashed lines are used to define the conditions relevant to the ASTM¹⁹ and ISRM⁴ standards.

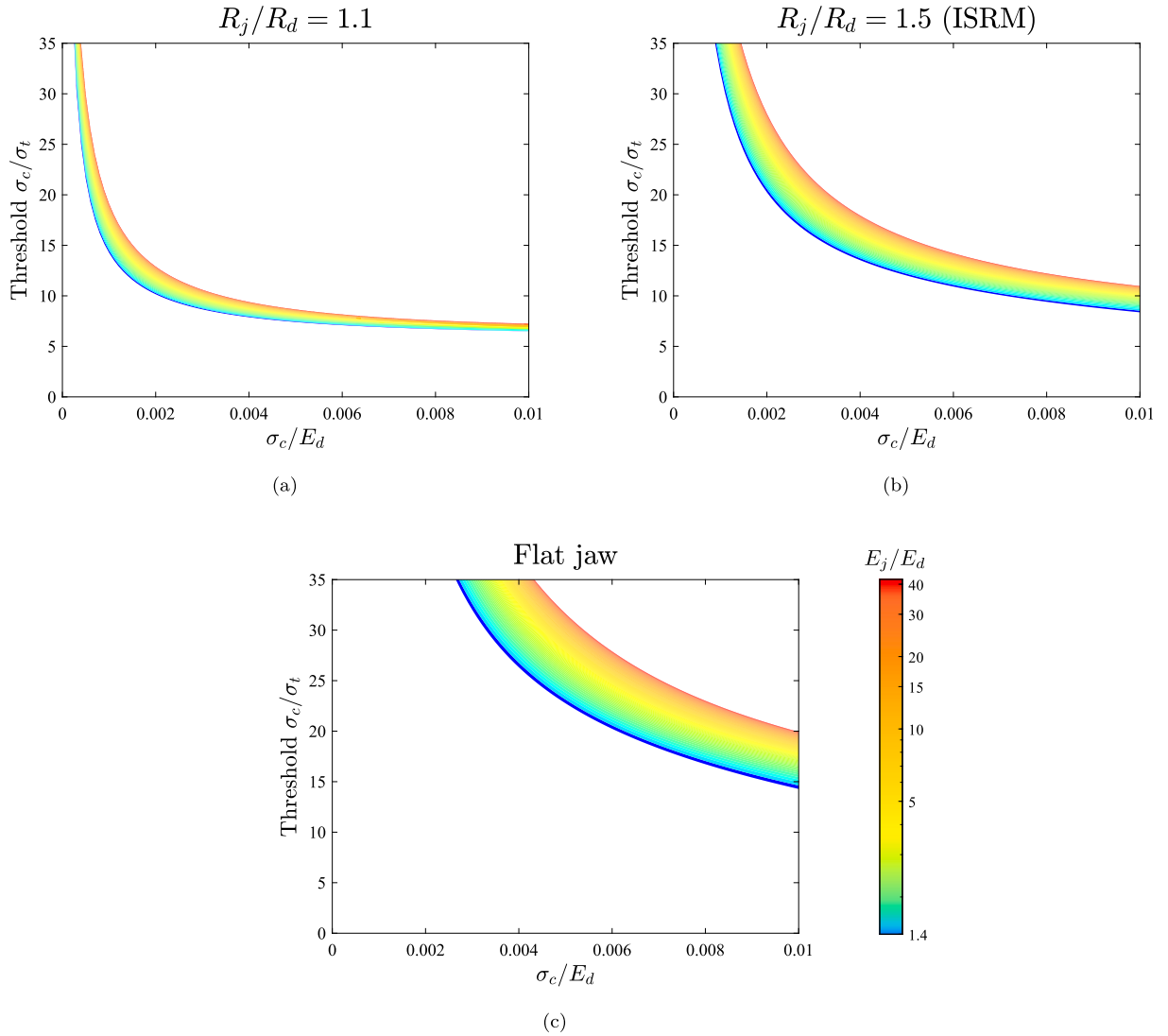


Fig. 11. Maps to assess if cracking nucleates at the centre. Influence of the elastic modulus of the material on the minimum acceptable ratio of compressive-to-tensile strength for (a) $R_j/R_d = 1.1$ (a low jaw radius), (b) $R_j/R_d = 1.5$ (the ISRM configuration), and (c) flat jaws (the ASTM configuration). The disk's Poisson's ratio equals $\nu_d = 0.2$.

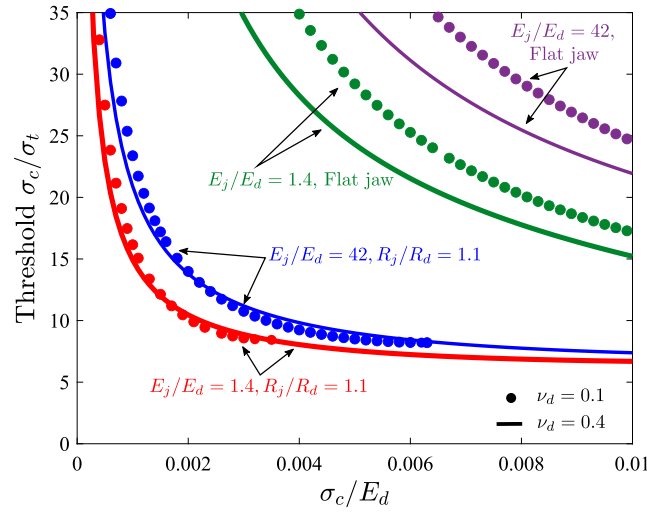


Fig. 12. Maps to assess if cracking nucleates at the centre. Influence of the Poisson ratio of the material on the minimum acceptable ratio of compressive-to-tensile strength. Results are obtained for the lower and upper bounds of ν_d (0.1, 0.4), E_j/E_d (1.4, 42) and R_j/R_d (1.1, 100).

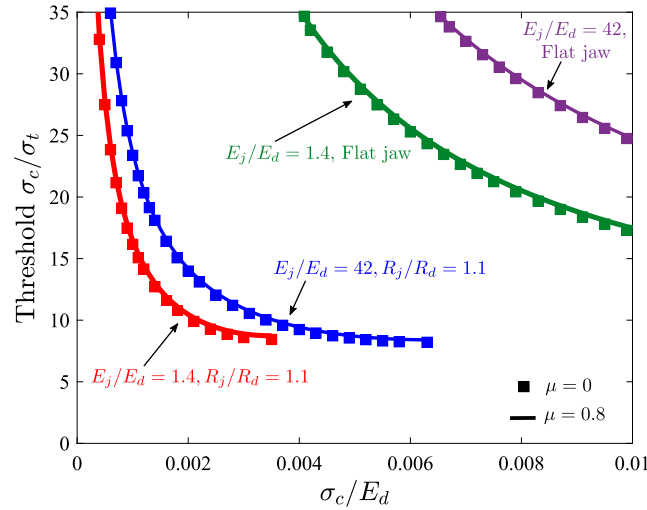


Fig. 13. Maps to assess if cracking nucleates at the centre. Influence of friction on the minimum acceptable ratio of compressive-to-tensile strength. Results are obtained without friction and for a friction coefficient of $\mu = 0.8$, for the lower and upper bounds of E_j/E_d (1.4, 42) and R_j/R_d (1.1, 100). The disk's Poisson's ratio equals $\nu_d = 0.1$.

5. A protocol for evaluating the validity of the Brazilian test

Identifying experimentally the location of crack nucleation in the Brazilian split test is hindered by the brittle behaviour of rocks; theoretical endeavours are needed to map the conditions of validity of the Brazilian test. The generalised Griffith criterion provides a suitable platform to achieve this as its failure envelope is given by two material properties: the tensile strength σ_t , which is estimated from the Brazilian test, and the compressive strength σ_c , which can be measured independently. In the following, we use the maps presented in Section 4 to provide a protocol to assess the validity of the Brazilian test as a function of the material and testing parameters. This is illustrated with examples of valid and invalid tests taken from the literature.

The protocol is a two-step process. First, one has to determine what is the maximum principal stress at the centre of the disk and second, one has to assess if cracking nucleated at the disk centre or elsewhere. Hondros's equations provide an estimate for the first step, but we have seen in Section 4.2 that these can be inaccurate. Thus, it is suggested that the maps provided in Section 4.2 and in the Supplementary Material are used instead to accurately determine the stress state at the disk centre. This corresponds with the material tensile strength ($\sigma_1 = \sigma_t$) if cracking initiated at the centre. The location of crack initiation is

assessed by using the maps presented in Section 4.3; since σ_c and E_d are known (they can be measured independently) we can estimate what is the admissible compressive-to-tensile strength ratio σ_c/σ_t for a choice of jaw radius R_j/R_d . If the magnitude of σ_c/σ_t resulting from the test is below this admissible threshold, then the test is invalid as cracking has nucleated outside of the centre of the disk. Alternatively, one can use this information before the test, using approximate expected values of σ_t (e.g., taken from the literature) to decide what is the most suitable testing geometry (R_j/R_d).

The protocol is exemplified with two examples of valid and invalid tests, taken from the literature. Specifically, we take as case studies the experiments by Sun and Wu³² on sandstone using the ISRM test configuration and the work by Duevel and Haimson³³ on granite, also using the ISRM recommended testing geometry. In both cases the jaws were made of steel, with elastic properties $E_j = 210$ GPa and $\nu_j = 0.3$. For the sandstone tested in Ref. 32, the reported elastic properties are $E_d = 19.15$ GPa and $\nu_d = 0.17$ and the material compressive strength is $\sigma_c = 99.93$ MPa. For the pink Lac du Bonnet granite study by Duevel and Haimson, the elastic properties are given by $E_d = 74.2$ GPa and $\nu_d = 0.25$, while the compressive strength was found to be $\sigma_c = 219$ MPa.^{33,34} The Brazilian tests conducted in Ref. 32 and Ref. 33 led to tensile strengths of $\sigma_t = 7.51$ MPa and $\sigma_t = 11.4$ MPa, respectively. Following

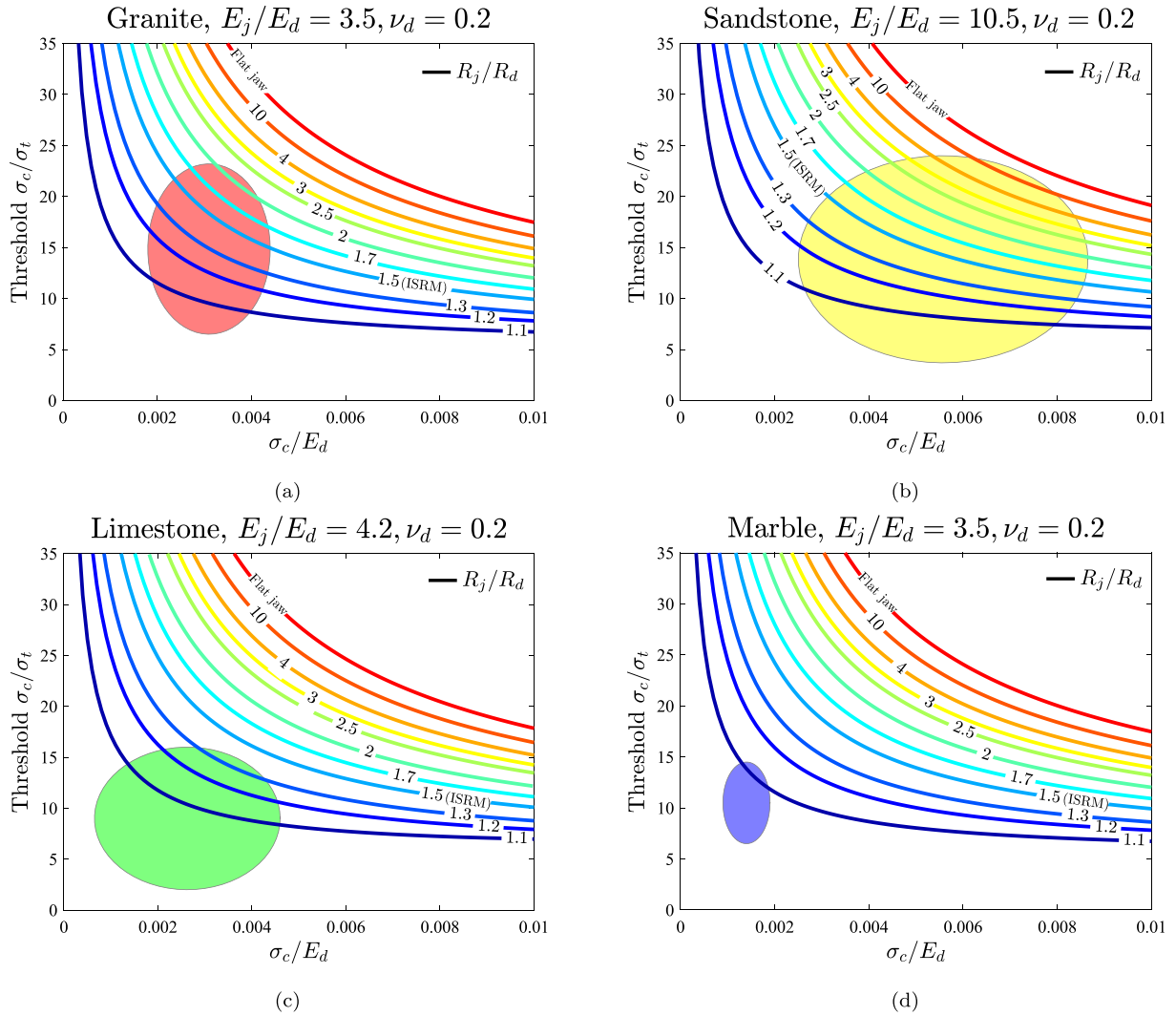


Fig. 14. Maps to assess if cracking nucleates at the centre: application to: (a) granite, (b) sandstone, (c) limestone, and (d) marble. The figure shows admissible compressive-to-tensile strength ratios as a function of the jaw radius (R_j/R_d) for the material properties of: (a) granite ($E_j/E_d = 1.5$, $\nu_d = 0.2$), (b) sandstone ($E_j/E_d = 10.5$, $\nu_d = 0.2$), (c) limestone ($E_j/E_d = 4.2$, $\nu_d = 0.2$), and (d) marble ($E_j/E_d = 3.5$, $\nu_d = 0.2$). Also, the domain of relevance of each material in a σ_c/σ_t vs σ_c/E_d plot is shown superimposed, as extracted from the GRANTA Material library.²³

the protocol presented above, we shall start by assessing the stress state at the disk centre at the critical load.

As described above, the first step lies in finding the maximum principal stress σ_1 at the centre for the critical applied load. Fig. 15 shows the maps presented in Section 4.2 particularised for the two case studies considered here: a sandstone with $E_j/E_d = 10.96$ and $\nu_d = 0.17$ (Fig. 15a) and a granite with $E_j/E_d = 2.83$ and $\nu_d = 0.25$ (Fig. 15b). The results of Fig. 15 reveal that, while in both case studies the stress state in the disk centre is not described by the point load equation, this approximation provides a good estimate. In the case of the sandstone study by Sun and Wu³² the error relative to Eq. (2) is below 0.5% while in the granite experiment by Duevel and Haimson³³ the error is roughly 0.2%. As shown in the figure, a better approximation can be obtained with flat jaws. In any case, Fig. 15 provides a way of obtaining an accurate estimate of the maximum principal stress at the disk centre, which equals $\sigma_1 = 7.47$ and $\sigma_1 = 11.38$ MPa for, respectively, the sandstone and the granite under consideration. These magnitudes

correspond to the material tensile strengths, provided that cracking nucleates at the disk centre.

The second and last step involves assessing the crack nucleation location. For the test to be valid, cracking must begin from the disk centre and, following the Griffith's generalised criterion, this will only happen if the compressive-to-tensile strength ratio is above the threshold of admissible values. Thus, given that σ_c and E_d are known, we can take the σ_t value obtained from the experiment in step 1 and see where the experimental data point lies in the maps presented in Section 4.3; this is done in Fig. 16 for both case studies and the testing geometries recommended by ASTM and ISRM, being the latter the one used in the tests.

The results of Fig. 16 show that while the granite study of Duevel and Haimson³³ provides a valid estimate of the material tensile strength, this is not the case for the sandstone experiment of Sun and Wu.³² The experimental data point lies below the contour corresponding to the testing geometry employed ($R_j/R_d = 1.5$), suggesting that

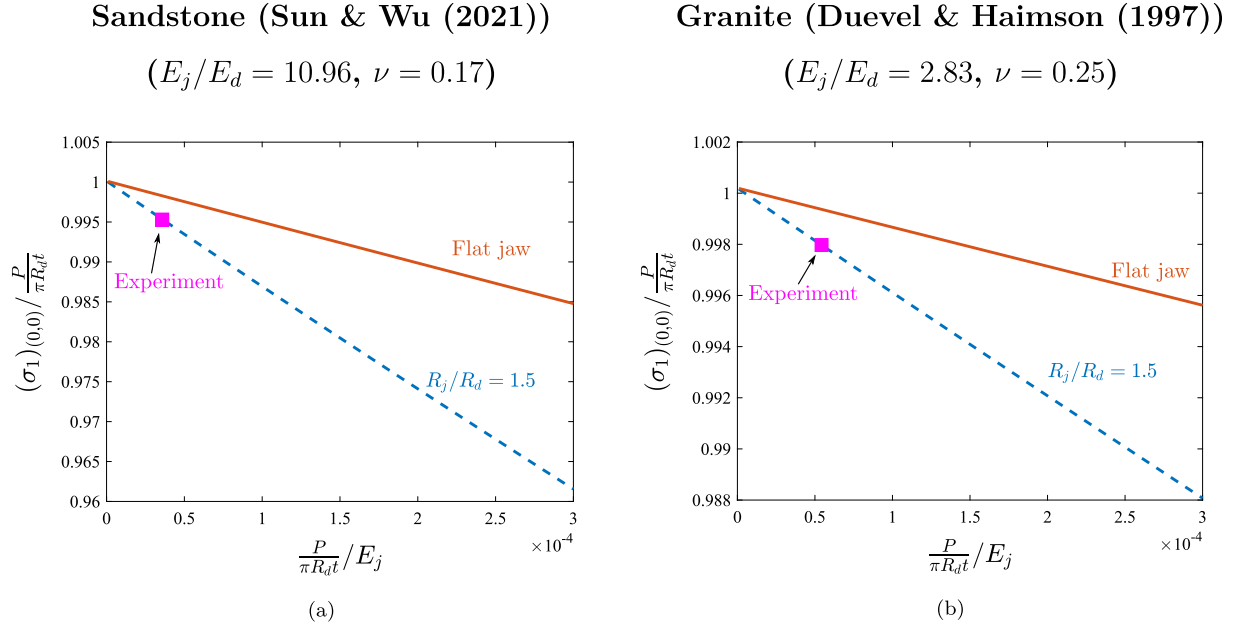


Fig. 15. A protocol for assessing the validity of the Brazilian test. Step 1 - evaluating the stress state at the disk centre for (a) the sandstone tested by Sun and Wu,³² and (b) the granite tested by Duevel and Haimson.³³ The material properties and critical load are $E_j/E_d = 10.96$, $\nu_d = 0.17$ and $P/(\pi R_d t) = 0.0000357E_j$ for (a), and $E_j/E_d = 2.83$, $\nu_d = 0.25$ and $P/(\pi R_d t) = 0.000054E_j$ for (b). The maps provided in Section 4.2 and the Supplementary Material are particularised for the two case studies under consideration and the ISRM ($R_j/R_d = 1.5$) and ASTM (flat jaws) testing configurations.

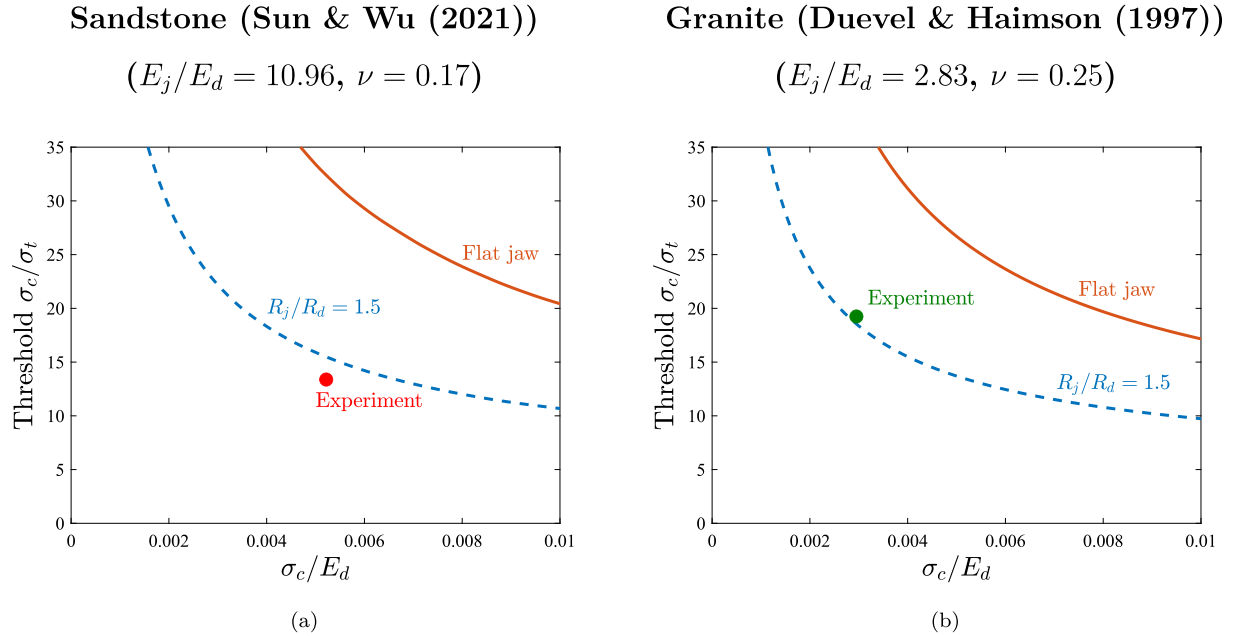


Fig. 16. A protocol for assessing the validity of the Brazilian test. Step 2 - evaluating the crack nucleation location for (a) the sandstone tested by Sun and Wu,³² and (b) the granite tested by Duevel and Haimson.³³ The material properties are $E_j/E_d = 10.96$, $\nu_d = 0.17$, $\sigma_c/E_d = 0.0052$ and $\sigma_c/\sigma_t = 13.37$ for (a), and $E_j/E_d = 2.83$, $\nu_d = 0.25$, $\sigma_c/E_d = 0.00295$ and $\sigma_c/\sigma_t = 19.24$ for (b). The maps provided in Section 4.3 and the Supplementary Material are particularised for the two case studies under consideration and the ISRM ($R_j/R_d = 1.5$) and ASTM (flat jaws) testing configurations. The admissible compression-to-tensile strengths establishes the threshold below which cracking initiates outside of the disk centre and the test becomes invalid.

cracking has initiated outside of the centre of the sample. This was also inferred from active and passive ultrasonic techniques in the study by Sun and Wu³², who concluded that cracking had initiated close to the jaws. Their comprehensive analysis, including numerical and experimental analysis of multiple testing configurations, showcased the limitations of the Brazilian test. The protocol and maps provided here (see also the Supplementary Material and Appendix A) enable establishing the conditions where the Brazilian test is valid, upon assuming that crack propagation is well approximated by the generalised Griffith criterion.

6. Conclusions

We have combined the generalised Griffith criterion and finite element analysis to theoretically assess the validity of the Brazilian split test. Maps have been provided to evaluate, as a function of material properties and test geometry, the fulfilment of the two assumptions inherent to the indirect estimate of the material tensile strength provided by the Brazilian test; that (i) the load is related to the maximum principal stress at the disk centre through Hondros's equations, and that (ii) cracking starts at the centre of the sample. The use of the generalised Griffith criterion enables assessing (ii) using a failure envelope that is solely a function of two material properties that can be independently measured: the tensile (σ_t) and compressive (σ_c) strengths. Our main findings are the following:

- For relevant contact angles, there is a noticeable deviation from the stress solution for a point load. However, the error remains small (below 5%) for a wide range of rock-like materials if flat or large-radius jaws are used.
- The use of the Hondros's stress solution for a uniformly distributed load ensures that the error does not exceed 4% for relevant ranges of stiffness mismatch and jaw radius. However, unlike the maps provided, requires an experimental characterisation of the contact angle at failure.
- The use of jaws with large radii favours the initiation of cracking in the compressive region, far from the disk centre, making the test invalid.
- The location of crack initiation is particularly sensitive to the testing geometry and, to a lesser degree, to the stiffness of the sample. Poisson's ratio plays a negligible role in jaws with a small radius but has an effect in the case of flat jaws. No influence of friction is observed.
- The analysis of the main classes of rocks reveals that the Brazilian test is not a suitable experiment for a wide range of materials. Only a small set of marbles and limestones (those with high σ_c/σ_t) can be adequately characterised and this requires the use of jaws with small radii. On the other hand, large-radius jaws can be used to test a range of granites and sandstones. The ISRM configuration ($R_j/R_d = 1.5$) appears to be solely suitable for these two latter classes of rocks, while the ASTM test geometry (flat jaws) was found to be unsuited to provide a valid estimate of tensile strength for any of the rock-like materials considered.

These findings suggest that the regimes of validity of the Brazilian test are much smaller than previously thought. To overcome these shortcomings and determine the range of conditions that lead to a valid Brazilian test, we have provided:

- Maps that relate the critical load with the stress state at the disk centre. These allow for accurately estimating the tensile strength without the need of using the approximation provided by the Hondros's equations.
- Maps that quantify the admissible compression-to-tensile strength ratios above which cracking initiates at the centre of the disk. These allow determining if the test is valid *a posteriori* or making *a priori* decisions of adequate test geometries based on expected σ_t values.
- A two-step protocol that will allow experimentalists to determine the validity of the test and accurately estimate the material tensile strength. The protocol is demonstrated with examples of valid and invalid tests from the literature. To facilitate uptake, this is encapsulated into a MATLAB App with an easy user interface.

Declaration of competing interest

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

Data availability

Data will be made available on request.

Acknowledgements

The authors acknowledge financial support from the Ministry of Science, Innovation and Universities of Spain through grant PGC2018-099695-B-I00. E. Martínez-Pañeda additionally acknowledges financial support from the Royal Commission for the 1851 Exhibition, UK through their Research Fellowship programme (RF496/2018).

Appendix A. BrazVal: A MATLAB App to assess the validity of the Brazilian test

A Matlab App is provided to facilitate the assessment of the validity of the Brazilian test, as per the Griffith generalised criterion and the analysis described in this manuscript. As shown in Fig. A.1, the MATLAB App contains a simple graphical user interface where the user provides as input variables the parameters related to the disk sample (radius R_d , Young's modulus E_d , Poisson's ratio ν_d , compressive strength σ_c and thickness t) and to the jaws (radius R_j , Young's modulus E_j , Poisson's ratio ν_j), as well as the critical load measured P_c . Upon clicking the button Run, the App provides the material tensile strength σ_t . If the test is deemed invalid, the message INVALID will be shown instead. In addition, the App provides the user with the tensile stress estimate based on Eq. (2), the actual tensile stress at the disk centre (which will coincide with σ_t if the test is valid) and the maximum allowable tensile strength, as determined from the threshold σ_c/σ_t ratio that ensures that cracking nucleates earlier at the disk centre than elsewhere.

The information provided by the App is based on a data grid generated by performing finite element calculations such as those described in Section 4. For scenarios for which data points do not exist, an estimate is attained by using linear interpolation (MATLAB's function `griddedInterpolant`). The App can be downloaded from www.empaneda.com/codes

Disk properties and parameters

Radius R_d (mm)

Young's modulus E_d (GPa)

Poisson's ratio ν_d

Compressive strength σ_c (MPa)

Thickness t (mm)

Jaw Properties

Radius R_j (mm)

Young's modulus E_j (GPa)

Poisson's ratio ν_j

Brazilian test Result

Critical load P_c (N)

Result

$\frac{P}{\pi R_d t}$

Tensile Stress at the Center of Disk $(\sigma_3)_c$ (MPa)

Maximum Allowable Tensile Strength $(\sigma_t)_a$ (MPa)

Tensile Strength σ_t (MPa)

Run

Fig. A.1. Graphical User Interface (GUI) of BrazVal, a MATLAB App to assess the validity of the Brazilian test, as a function of material and testing parameters. The App can be downloaded from www.empaneda.com/codes.

Appendix B. Supplementary data

Supplementary material related to this article can be found online at <https://doi.org/10.1016/j.ijrmms.2022.105227>.

References

- Li D, Wong LNY. The Brazilian disc test for rock mechanics applications: Review and new insights. *Rock Mech Rock Eng.* 2013;46(2):269–287.
- Carneiro F. A new method to determine the tensile strength of concrete. In: *Proceedings of the 5th Meeting of the Brazilian Association for Technical Rules, Section 3d.* 1943.
- Akazawa T. New test method for evaluating internal stress due to compression of concrete. *J Japan Soc Civ Eng.* 1943;29:777–787.
- Bieniawski ZT, Hawkes I. Suggested methods for determining tensile strength of rock materials. *Int J Rock Mech Min Sci.* 1978;15(3):99–103.
- Hondros G. The evaluation of Poisson's ratio and the modulus of materials of a low tensile resistance by the Brazilian (indirect tensile) test with particular reference to concrete. *Aust J Appl Sci.* 1959;10(3):243–268.
- Timoshenko S, Goodier J. *Theory of Elasticity.* McGraw-Hill; 1951.
- Fairhurst C. On the validity of the 'Brazilian' test for brittle materials. *Int J Rock Mech Min Sci.* 1964;1(4):535–546.
- Hudson JA, Brown ET, Rummel F. The controlled failure of rock discs and rings loaded in diametral compression. *Int J Rock Mech Min Sci.* 1972;9(2):241–248.
- Alvarez-Fernandez MI, Garcia-Fernandez CC, Gonzalez-Nicieza C, Guerrero-Miguel DJ. Effect of the contact angle in the failure pattern in slate under diametral compression. *Rock Mech Rock Eng.* 2020;53(5):2123–2139.
- Markides CF, Kourkoulis SK. The influence of jaw's curvature on the results of the Brazilian disc test. *J Rock Mech Geotech Eng.* 2016;8(2):127–146.
- Gutiérrez-Moizant R, Ramírez-Berasategui M, Sánchez-Sanz S, Santos-Cuadros S. Experimental verification of the boundary conditions in the success of the Brazilian test with loading arcs, an uncertainty approach using concrete disks. *Int J Rock Mech Min Sci.* 2020;132:104380.
- Bouali MF, Bouassida M. Numerical simulation of the effect of loading angle on initial cracks position point: Application to the Brazilian test. *Appl Sci.* 2021;11(8).
- Garcia-Fernandez CC, Gonzalez-Nicieza C, Alvarez-Fernandez MI, Gutierrez-Moizant RA. Analytical and experimental study of failure onset during a Brazilian test. *Int J Rock Mech Min Sci.* 2018;103:254–265.
- Zhao Z, Sun W, Chen S, Yin D, Liu H, Chen B. Determination of critical criterion of tensile-shear failure in Brazilian disc based on theoretical analysis and meso-macro numerical simulation. *Comput Geotech.* 2021;134:104096.
- Aliabadian Z, Zhao GF, Russell AR. Failure, crack initiation and the tensile strength of transversely isotropic rock using the Brazilian test. *Int J Rock Mech Min Sci.* 2019;122:104073.
- Erarslan N, Liang ZZ, Williams DJ. Experimental and numerical studies on determination of indirect tensile strength of rocks. *Rock Mech Rock Eng.* 2012;45(5):739–751.
- Yu H, Andersen DH, He J, Zhang Z. Is it possible to measure the tensile strength and fracture toughness simultaneously using flattened Brazilian disk? *Eng Fract Mech.* 2021;247:107633.
- Komurlu E, Kesimal A. Evaluation of indirect tensile strength of rocks using different types of jaws. *Rock Mech Rock Eng.* 2015;48(4):1723–1730.
- ASTM D3697 Standard Test Method for Splitting Tensile Strength of Intact Rock Core Specimens. West Conshohocken, PA, ASTM International.
- Griffith AA. The theory of rupture. In: *Proc. First International Congress for Applied Mechanics.* 1924:55–63.

21. Jaeger J, Cook N, Zimmerman R. *Fundamentals of Rock Mechanics*. Oxford, UK: Blackwell Publishing; 2009.
22. Hoek E, Martin CD. Fracture initiation and propagation in intact rock - A review. *J Rock Mech Geotech Eng*. 2014;6(4):287–300.
23. *Ansys Granta EduPack*. Cambridge, UK: ANSYS Inc.; 2021.
24. Lin H, Xiong W, Zhong W, Xia C. Location of the crack initiation points in the Brazilian disc test. *Geotech Geol Eng*. 2014;32(5):1339–1345.
25. Yuan R, Shen B. Numerical modelling of the contact condition of a Brazilian disk test and its influence on the tensile strength of rock. *Int J Rock Mech Min Sci*. 2017;93(December 2015):54–65.
26. Papazafeiropoulos G, Muñoz-Calvente M, Martínez-Pañeda E. Abaqus2Matlab: A suitable tool for finite element post-processing. *Adv Eng Softw*. 2017;105:9–16.
27. Lavrov A, Vervoort A. Theoretical treatment of tangential loading effects on the Brazilian test stress distribution. *Int J Rock Mech Min Sci*. 2002;39(2):275–283.
28. Markides CF, Pazis DN, Kourkoulis SK. Influence of friction on the stress field of the Brazilian tensile test. *Rock Mech Rock Eng*. 2011;44(1):113–119.
29. Markides CF, Pazis DN, Kourkoulis SK. The Brazilian disc under non-uniform distribution of radial pressure and friction. *Int J Rock Mech Min Sci*. 2012;50:47–55.
30. Markides CF, Kourkoulis SK. Naturally accepted boundary conditions for the Brazilian disc test and the corresponding stress field. *Rock Mech Rock Eng*. 2013;46(5):959–980.
31. Hooper JA. The failure of glass cylinders in diametral compression. *J Mech Phys Solids*. 1971;19(4):179–188.
32. Sun W, Wu S. A study of crack initiation and source mechanism in the Brazilian test based on moment tensor. *Eng Fract Mech*. 2021;246:107622.
33. Duevel B, Haimson B. Mechanical characterization of pink Lac du Bonnet granite: Evidence on nonlinearity and anisotropy. *Int J Rock Mech Min Sci Geomech Abstracts*. 1997;34(3–4):543.
34. Cai M. Practical estimates of tensile strength and Hoek-Brown strength parameter m_i of brittle rocks. *Rock Mech Rock Eng*. 2010;43(2):167–184.